

# JORDAHL® Technical Information

## JTB-uni Channels for Windows & Window Wall



JORDAHL® JTB-uni anchor channels are used with self-tapping screws, and are an ideal solution for window wall and window connections to concrete. They provide the following user benefits:

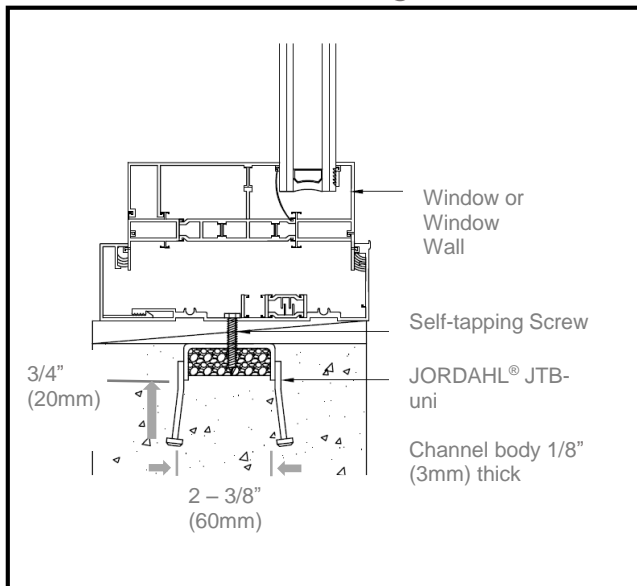
- Fast and reliable connections to concrete.
- Two dimensional connection adjustment.
- No drilling of concrete - No dust. No concrete cracking or reinforcement conflict.
- No welding - No spark damage or fires.
- Easy placement

Standard channel length: 12" (300mm) with 4 anchors  
Standard finish: Pre-galvanized carbon steel

Stainless steel and longer lengths are also available.



## Installation and Design Data



JORDAHL® JTB-uni channels can be used to transfer wind loads into the concrete slab or architectural precast panels according to the data provided below. The design engineer should also check that the capacity of the selected self-tapping fasteners are sufficient to transfer the applied loads.

Single connections or double connections (at minimum 6" spacing) can be made at any position greater than 1-1/2" from the ends of each channel, providing that the maximum capacity of the channel is not exceeded.

JORDAHL® JTB-uni channels should be cast into concrete according to the minimum dimensions provided below. The concrete around the channel should be carefully compacted, and the channel should be straight and level with the surface of the concrete.

**Factored Nominal Strength (LRFD): †**

Shear  $\phi V_s = 1,700$  lbs (7.4 kN) assuming cracked and un-reinforced concrete at 2,500 psi, with minimum dimensions of 3" (75mm) edge distance and 7" (175mm) concrete depth.

The factors below and on the following page may be applied to adjust for different concrete conditions.

**Un-cracked concrete factor ( $\psi_0$ ):**

$$\psi_0 = 1.4$$

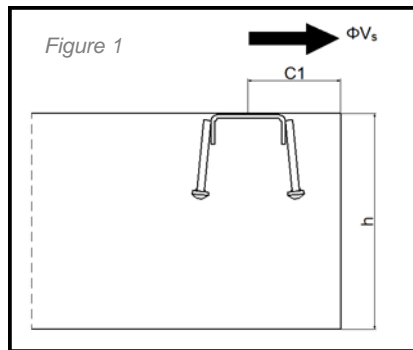
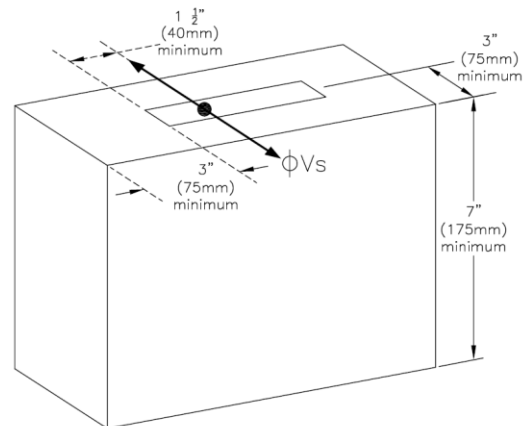
**Concrete strength factor ( $\psi_1$ ):**

$$\psi_1 = \sqrt{\frac{\text{Project } f'c' (4,500 \text{ psi max.})}{2,500 \text{ psi}}}$$

**Reduction Factor for Slab Connections ( $\psi_2$ ):**

$$0.56 \leq \psi_2 \leq 1.00 \text{ (see tables 1 \& 2)}$$

**Minimum Dimensions for Edge Connections**

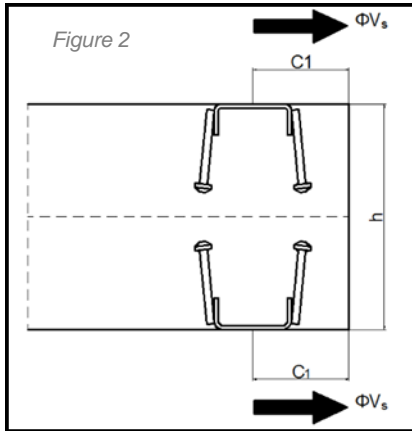


**Table 1 - Reduction Factors for Top of Slab/Panel Edge Connections**

Edge distance $C_1$ [in]	Concrete thickness $h$ [in]				
	5	5-1/2	6	6-1/2	7
	<b>Modification factor (<math>\psi_2 \leq 1</math>) for slab thickness <math>h \leq 7</math>"</b>				
3	0.78	0.84	0.89	0.93	1.00
3-1/2	0.88	0.94	1.00	1.00	1.00

† For calculations using Allowable Stress Design (ASD), the allowable shear capacity = 1,100 lbs (4.9 kN)

**Table 2 - Reduction Factors for Top and Bottom Slab Connections**



Edge distance C <sub>1</sub> [in]	Slab thickness h [in]								
	6	6-1/2	7	7-1/2	8	8-1/2	9	9-1/2	10
Modification factor ( $\psi_2 \leq 1$ ) for slab thickness $h \leq 10"$									
3	0.56	0.59	0.62	0.65	0.68	0.70	0.73	0.76	0.78
3-1/2	0.63	0.66	0.70	0.73	0.76	0.79	0.82	0.85	0.88
4	0.70	0.73	0.77	0.81	0.84	0.88	0.91	0.94	0.98
4-1/2	0.76	0.80	0.84	0.88	0.92	0.96	1.00	1.00	1.00
5	0.82	0.87	0.91	0.95	1.00	1.00	1.00	1.00	1.00

## Calculation Examples

### 1. Edge connection to architectural precast panel. (Fig. 1)

Design Conditions

Project  $fc' = 5,000$  psi

$C_1 = 3"$ ;  $h = >7"$

Design load = 2,500 lbs

Concrete un-cracked

Calculation

$$\psi_0 = 1.4; \psi_1 = \sqrt{\frac{\text{Project } fc' (4,500 \text{ psi max.})}{2,500 \text{ psi}}} = 1.34; \psi_2 = 1.00 \text{ (table 1)}$$

$$\text{Shear } \phi V_s = 1,700 \text{ lbs} \times 1.4 \times 1.34 \times 1.00 = 3,189 \text{ lbs}$$

$$\text{Shear } \phi V_s = 3,189 \text{ lbs} > \text{Design Load} - \text{OK}$$

### 2. Top and bottom slab connection. (Fig. 2)

Design Conditions

Project  $fc' = 4,300$  psi

$C_1 = 3"$ ;  $h = 8"$

Design load = 2,000 lbs

Concrete un-cracked

Calculation

$$\psi_0 = 1.4; \psi_1 = \sqrt{\frac{\text{Project } fc' (4,500 \text{ psi max.})}{2,500 \text{ psi}}} = 1.31; \psi_2 = 0.68 \text{ (table 2)}$$

$$\text{Shear } \phi V_s = 1,700 \text{ lbs} \times 1.4 \times 1.31 \times 0.68 = 2,120 \text{ lbs}$$

$$\text{Shear } \phi V_s = 2,120 \text{ lbs} > \text{Design Load} - \text{OK}$$

Our engineering and sales support team will be pleased to provide further specific assistance for your project, as required.